

**GENERAL NOTES**

**DESIGN LOADS**

|               |  |
|---------------|--|
| BUILDING CODE | CBC 2016 (BASED ON IBC 2015)                                     |
| LIVE LOADS    | 5 PSF  |
| SNOW LOAD     | 5 PSF  |
| WIND LOADS    | 115 MPH (3-SEC. GUST); EXPOSURE C; TOPOGRAPHIC FACTOR, Kzt = 1.0 |

- SPECIAL INSPECTION REQUIREMENTS SHALL FOLLOW THE ATTACHED SAMPLE TEST AND INSPECTION LIST (T & I LIST) APPROVED BY DSA. THE SHOP WELDING INSPECTION SHALL INCLUDE WELDING OF ALL STEEL MEMBERS AND IDENTIFICATION OF STEEL THROUGH MILL CERTIFICATE OR MATERIAL TESTING. UNCERTIFIED STEEL SHALL BE TESTED TO THE REQUIREMENTS OF CBC 2016 CHAPTER 17A. THE FIELD SPECIAL INSPECTION SHALL INCLUDE COMPRESSION CYLINDER TESTS FOR THE CONCRETE FOUNDATION.
- STRUCTURE SHALL BE IN THE LOCATION SHOWN ON THE SITE SPECIFIC DSA APPLICATION DRAWING.
- FOUNDATION DESIGN BASED ON CBC 2016, TABLE 1806A.2, SOIL CLASS 5 (ALLOWABLE FOUNDATION PRESSURE 1500 PSF)
- DESIGN PER FOLLOWING CODES: CBC 2016, ASCE 7-10, AISC 360-10, AISC 341-10, ACI 318-14, ASCE 55-10 & ASCE 19-10

**STRUCTURAL STEEL**

- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL BE CONDUCTED PER CBC 2016 SECTIONS 1704A, 1705A, 1705A.2, AND TABLE 1705A.2.1.
- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE SHADE STRUCTURES.
- ALL WORK SHALL CONFORM TO CBC 2016 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR)
- ALL GALVANIZED STEEL TUBE PRODUCTS MANUFACTURED BY ALLIED TUBE & CONDUIT FOR THIS STRUCTURE SHALL BE, AND CONFORM TO ASTM A500-10, GRADE "B", IN ITS ENTIRETY. TYPICAL MECHANICAL PROPERTIES ARE:  
ROUND TUBE 42,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS
- ALL STRUCTURAL SHAPES SHALL BE COLD FORMED HSS ASTM A500 GRADE B, UNLESS OTHERWISE NOTED. TYPICAL MECHANICAL PROPERTIES ACHIEVED FOR HSS PRODUCTS:  
SQUARE AND RECTANGULAR 46,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS  
ROUND PIPE 42,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS
- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.
- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.
- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/CWI INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COLD FORMED. AWS D1.8 SEISMIC SUPPLEMENT.
- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.
- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. FIELD CONNECTIONS SHALL BE AS INDICATED ON THE DRAWINGS (IF REQUIRED). ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" ER70S-X ELECTRODES UNLESS OTHERWISE NOTED. EITHER SMAW OR GMAW IS ACCEPTABLE.
- ALL STAINLESS STEEL BOLTS SHALL COMPLY WITH ASTM F-593, FYS = 60 KSI, FS = 95KSI. ALLOY GROUP 1 OR 2 ALL NUTS SHALL COMPLY WITH ASTM F-594 ALLOY GROUP 1 OR 2. REFERRING TO RCSC, ASTM F-593 IS NOT CONSIDERED AS HIGH STRENGTH BOLTS.
- ALL STRUCTURAL STEEL (ITEMS FROM NOTE 5) SHALL BE PAINTED WITH ONE SHOP COAT (2.5 TO 3.5 MILS THICK MIN) OF ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER TGIC (MANUFACTURED BY SHERWIN WILLIAMS OR TIGER DRYLAC). TO ACHIEVE OPTIMUM ADHESION, IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TGIC) SPECIFICATIONS SHALL BE AS FOLLOWS:  
- PENCIL HARDNESS (ASTM D-3363).  
- HUMIDITY (ASTM D-2247).  
- SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL. SOFTNESS.
- ALL STEEL ROUND TUBING (ITEMS FROM NOTE 4) SHALL BE TRIPLE COATED FOR RUST PROTECTION USING THE IN-LINE ELECTROPLATING COAT PROCESS. TUBING SHALL BE INTERNALLY COATED WITH ZINC AND ORGANIC COATINGS TO PREVENT CORROSION AS MANUFACTURED BY ALLIED TUBE & CONDUIT.
- COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION. ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329), OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

**CONCRETE SPECIFICATION**

- CONCRETE SHALL BE TESTED PER CBC 2016 SECTION 1903A & SHALL BE INSPECTED PER SECTION 1903A.
- CONCRETE TO BE F'c = 4500 PSI, TYPE V CEMENT, WATER/CEMENT RATIO OF 0.45, PER ACI 318-14 CHAPTER 5. REINFORCING STEEL TO BE Fy = 60000 PSI, MIN. GR. 60
- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 55 (GALVANIZED). ANCHOR BOLTS EMBEDMENT NEEDS TO BE AS FOLLOWS:  
A) ANCHOR BOLT Ø1 1/4" 30 IN (MINIMUM EMBEDMENT)
- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.
- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C1109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.

**FABRIC SPECIFICATION**

- FABRIC SHALL BE MANUFACTURED BY MULTIKNIT LTD. OR OTHER COMPANY WHO CAN MANUFACTURE FABRIC WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS.
- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR EVERY 12 HOURS.
- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHALL TO DSA AT SITE SPECIFIC INSTALLATION.
- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT. FABRICS SAMPLES OF THE SAME MATERIAL WHICH ARE MAINTAINED AT THE PROJECTS SITE SHALL BE TESTED TO BE IN COMPLIANCE WITH ASTM D5034 AND D2261. THE ANNUAL TESTING ON THE APPROVED PLANS SHALL BE COMPARED TO THE FABRIC SPECIFICATIONS INDICATED IN NOTE 1 OF "FABRIC SPECIFICATION" ON THE APPROVED PLANS. THE FABRIC SHALL BE REPLACED WHEN THE TEST RESULTS RETURN LESS THAN 50% OF THE ULTIMATE VALUES IN NOTE 1 OF "FABRIC SPECIFICATION".
- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED, FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.
- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. SHADE STRUCTURE SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

**AIRCRAFT CABLE**

- FOR FABRIC ATTACHMENT USE 3/8" 7x19 GALV. CABLE PER ASTM A1023A, ASTM 1023M-02, WITH A BREAKING STRENGTH VALUE OF 14,400 LBS. CABLE SHALL BE TENSIONED TO 250 LBS MINIMUM. THE MAXIMUM CALCULATED CABLE TENSION IS 2189 LB.
- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTENING VISITS AS REQUIRED.

**2016 CBC PC DESIGN NOTES**

|  |            |  |
|--|------------|--|
| FLOOR LIVE LOAD  | N/A        | 5 PSF                                  |
| ROOF LIVE LOAD   | RL         | 5 PSF                                  |
| ALLOWABLE SOIL PRESSURE:   |            |  |
| DL + LL (CONC FTG)   | 1500 PSF   |  |
| DL + LL + SEISMIC (CONC FTG)   | 1500 PSF   |  |
| LATERAL BEARING DESIGN VALUE   | 100 PSF/FT | BELOW NATURAL GRADE, PER TABLE 1806A.2 |
| TWO TIMES THE TABULAR VALUE IS USED (200 PSF/FT)   |            |  |
| PER CBC SECTION 1806A.3.4.   |            |  |
| ALLOWABLE PIER FRICTIONAL RESISTANCE 250 PSF MAXIMUM   |            |  |
| BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE).   |            |  |
| UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.  |            |  |
| ROOF SNOW LOAD   | 5 PSF      |  |
| FLOOD HAZARD AREA  | NO         |  |
| WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE. |            |  |
| WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-10, SECTION 27.4.3   |            |  |
| -ULTIMATE DESIGN WIND SPEED (3 SEC GUST)   | V          | 115 MPH                                |
| -WIND EXPOSURE FACTOR  | C          |  |
| -TOPOGRAPHIC FACTOR  | Kzt        | 1                                      |
| -RISK CATEGORY   | R          | 1                                      |
| -VELOCITY PRESSURE EXPOSURE COEFFICIENT  | Kz         | 0.85                                   |
| -VELOCITY PRESSURE   | qz         | 24.48 PSF                              |
| SEISMIC DESIGN:  |            |  |
| -SITE CLASS  | D          |  |
|  | SS         | 3.00g                                  |
|  | S1         | 1.389g                                 |
|  | SDS        | 2.00                                   |
|  | SD1        | 1.39                                   |
| -SPECTRAL RESPONSE COEFFICIENTS  |            |  |
| -LATERAL FORCE RESISTING SYSTEM G 2 ORDINARY CANTILEVERED COLUMN SYSTEM.   |            |  |
| -SEISMIC IMPORTANCE FACTOR   | I          | 1.0                                    |
| -DESIGN BASE SHEAR   | V          | 1699 LB                                |
| -SEISMIC RESPONSE COEFFICIENTS   | Cs         | 1.6                                    |
| -RESPONSE MODIFICATION FACTOR  | R          | 1.25                                   |
| -ANALYSIS PROCEDURE  |            | EQUIVALENT LATERAL FORCE               |
| -RISK CATEGORY   | II         |  |
| -SEISMIC DESIGN CATEGORY   | E          |  |
| -SITE COEFFICIENT CATEGORY   | Fv         | 1.5                                    |

GEOHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQ FT OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3.1.1 DO NOT REQUIRE A GEOHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

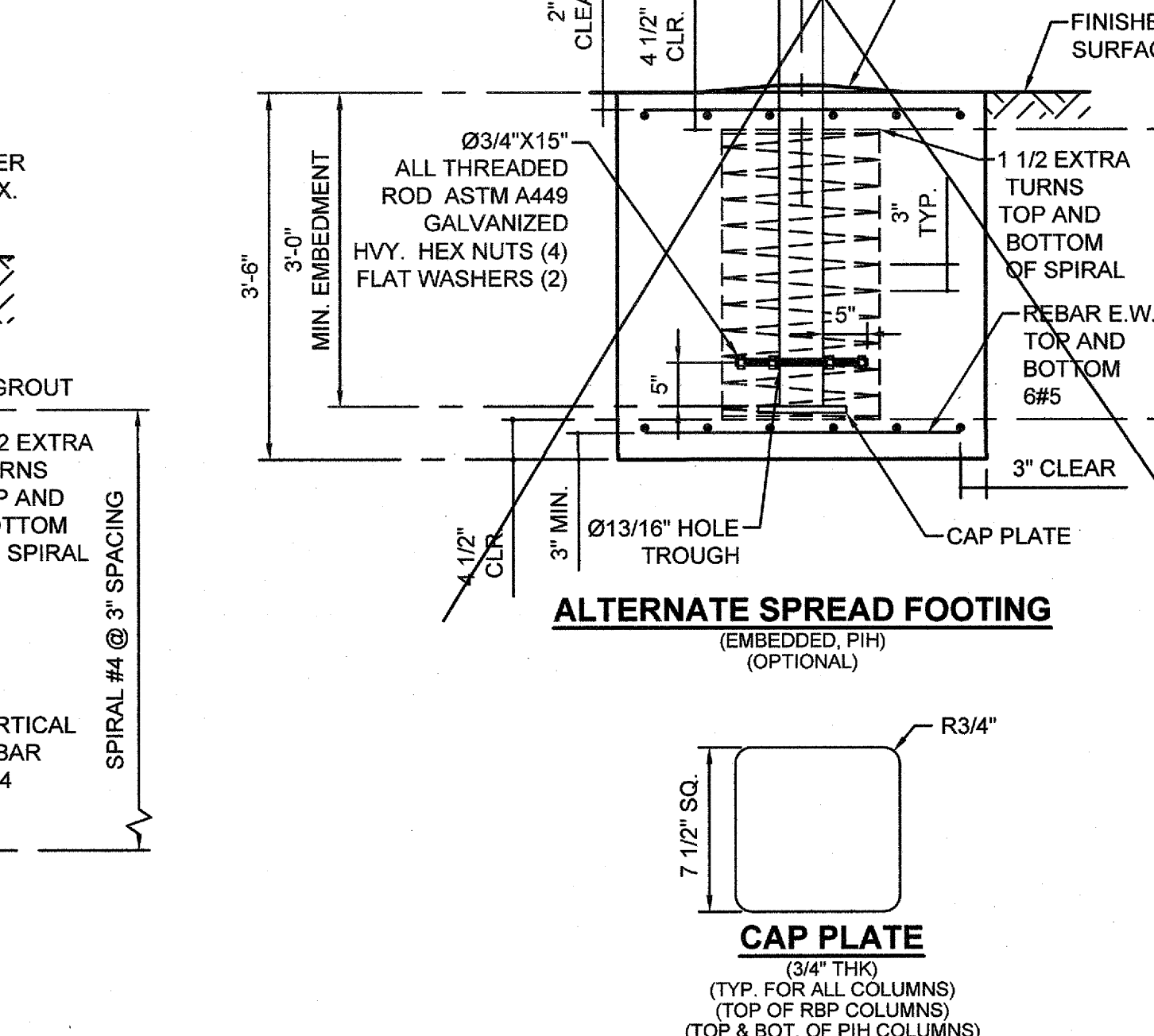
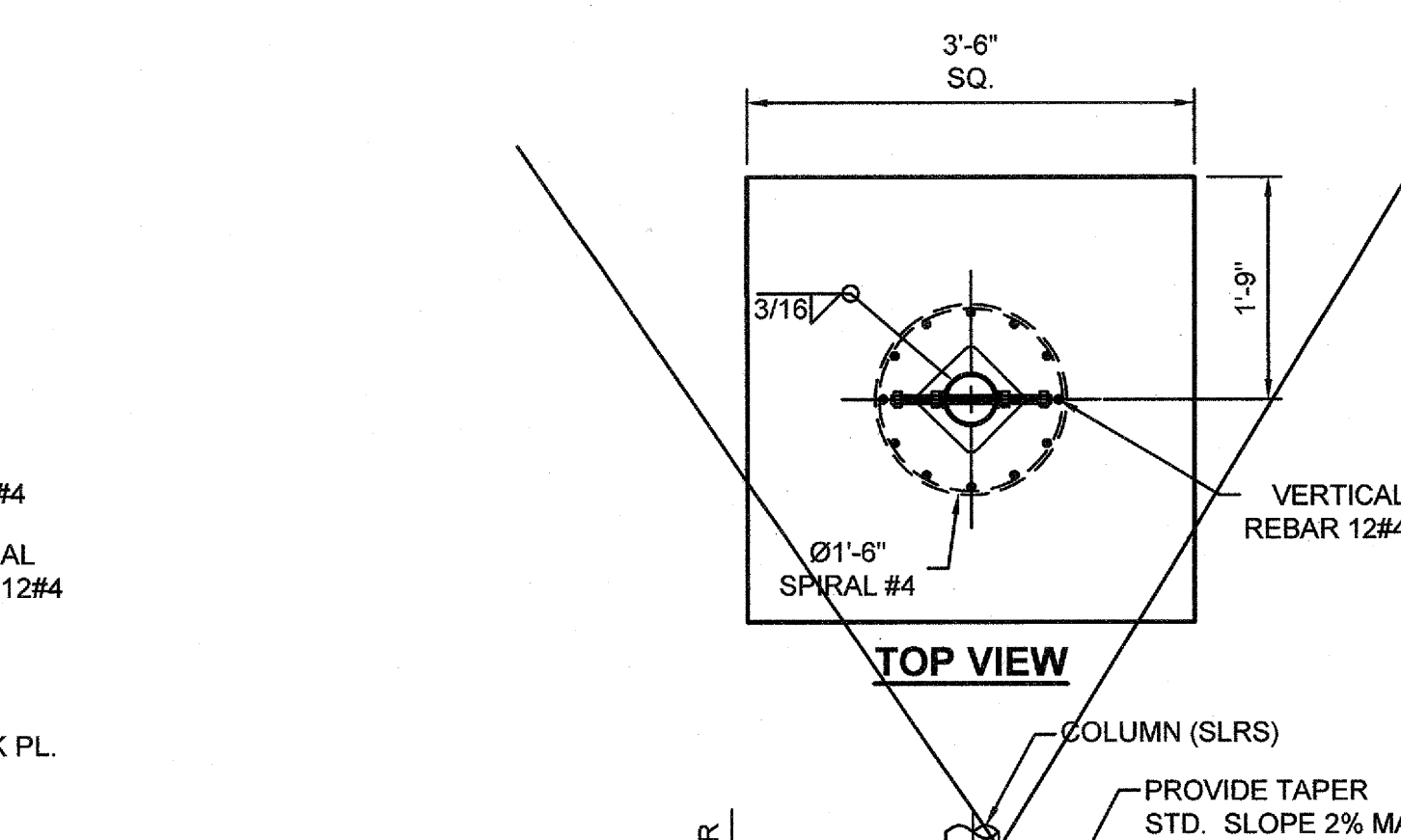
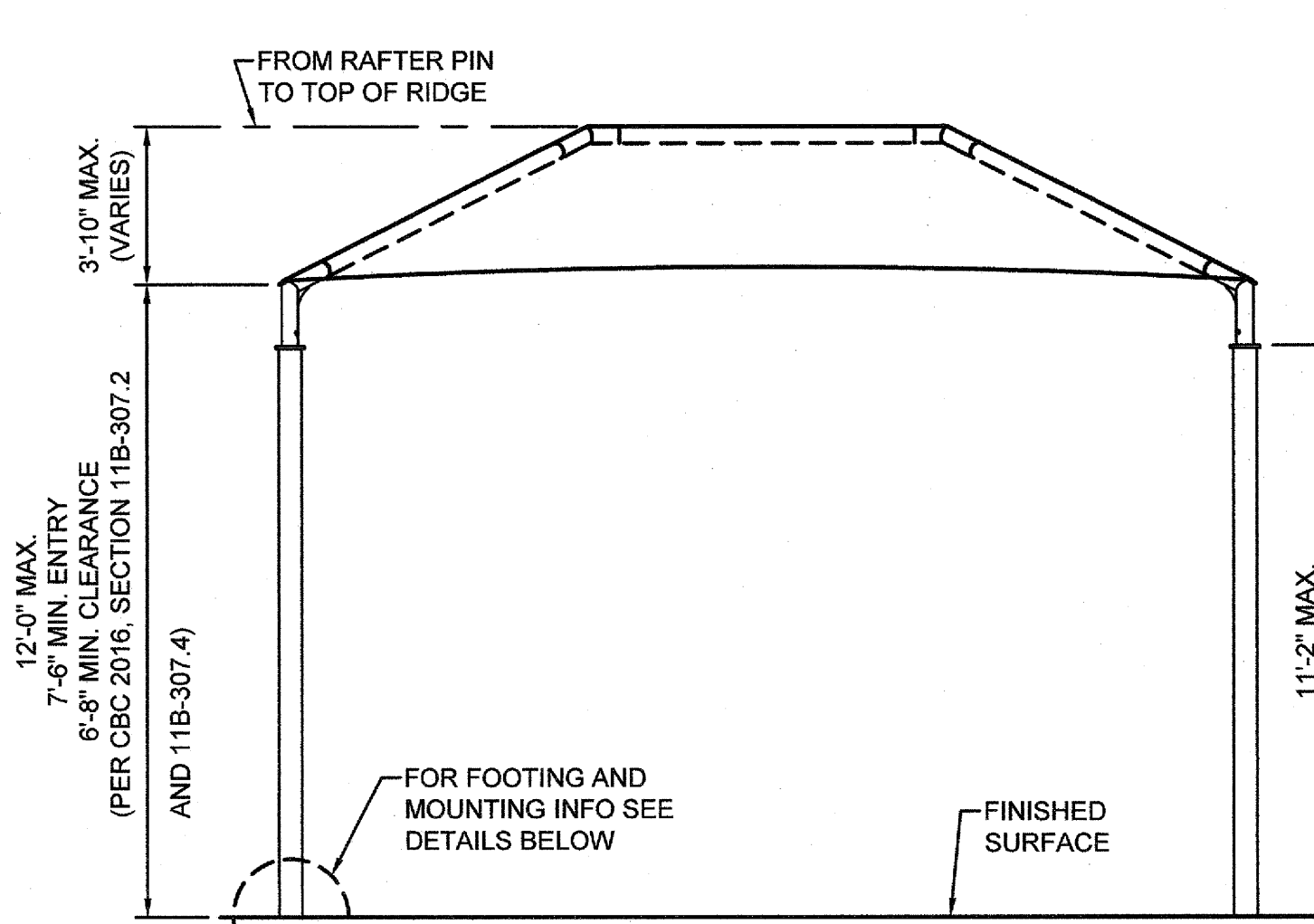
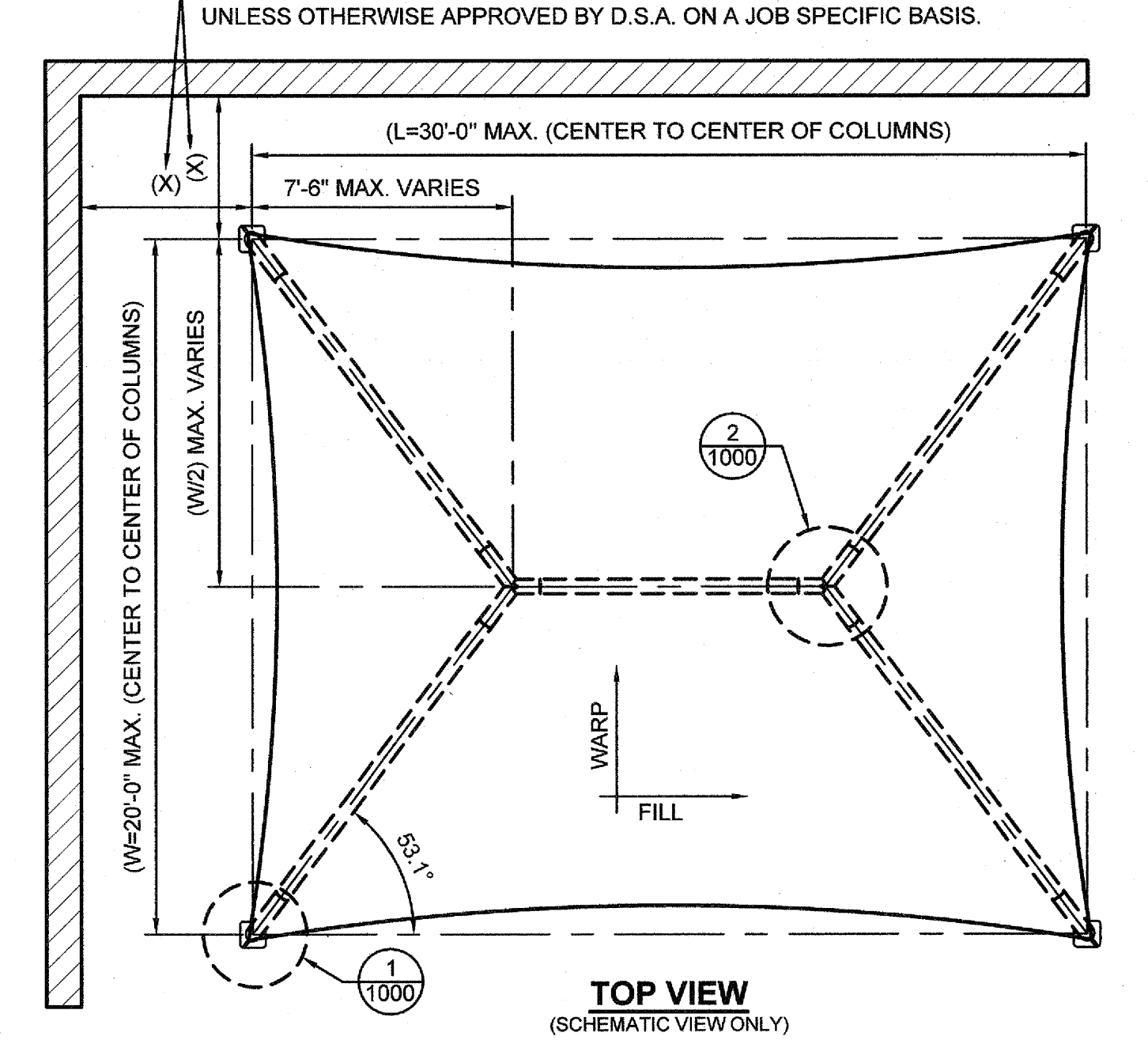
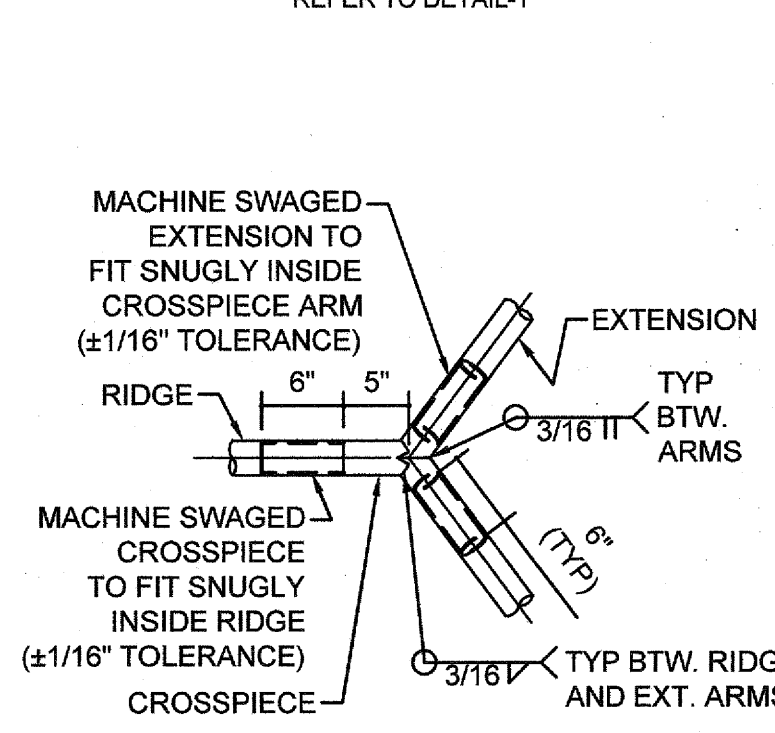
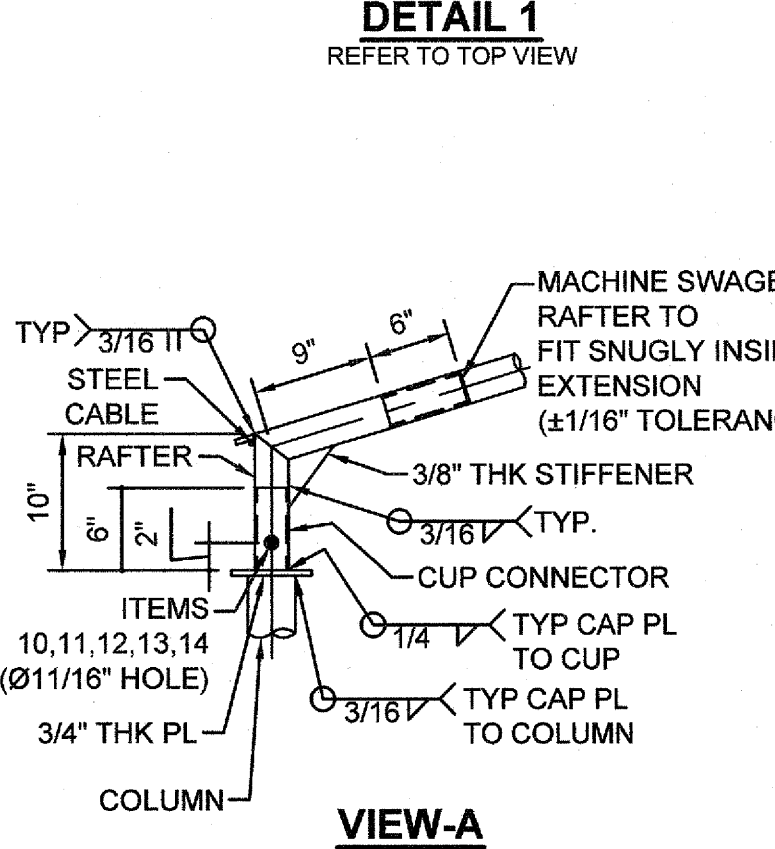
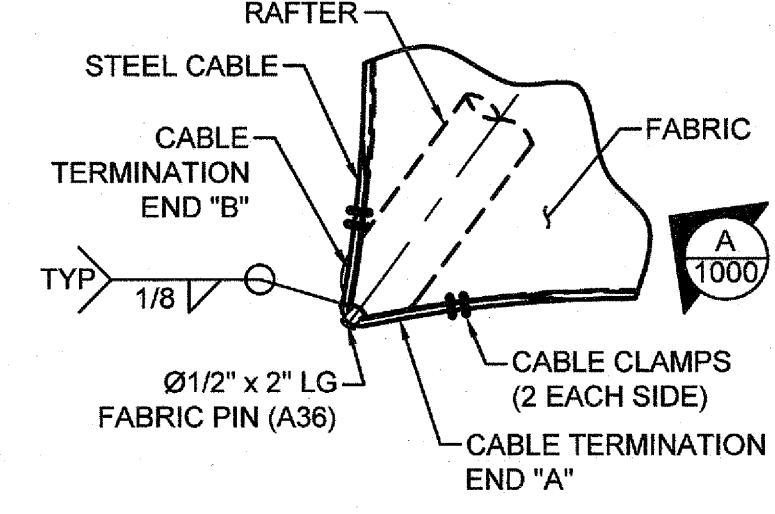
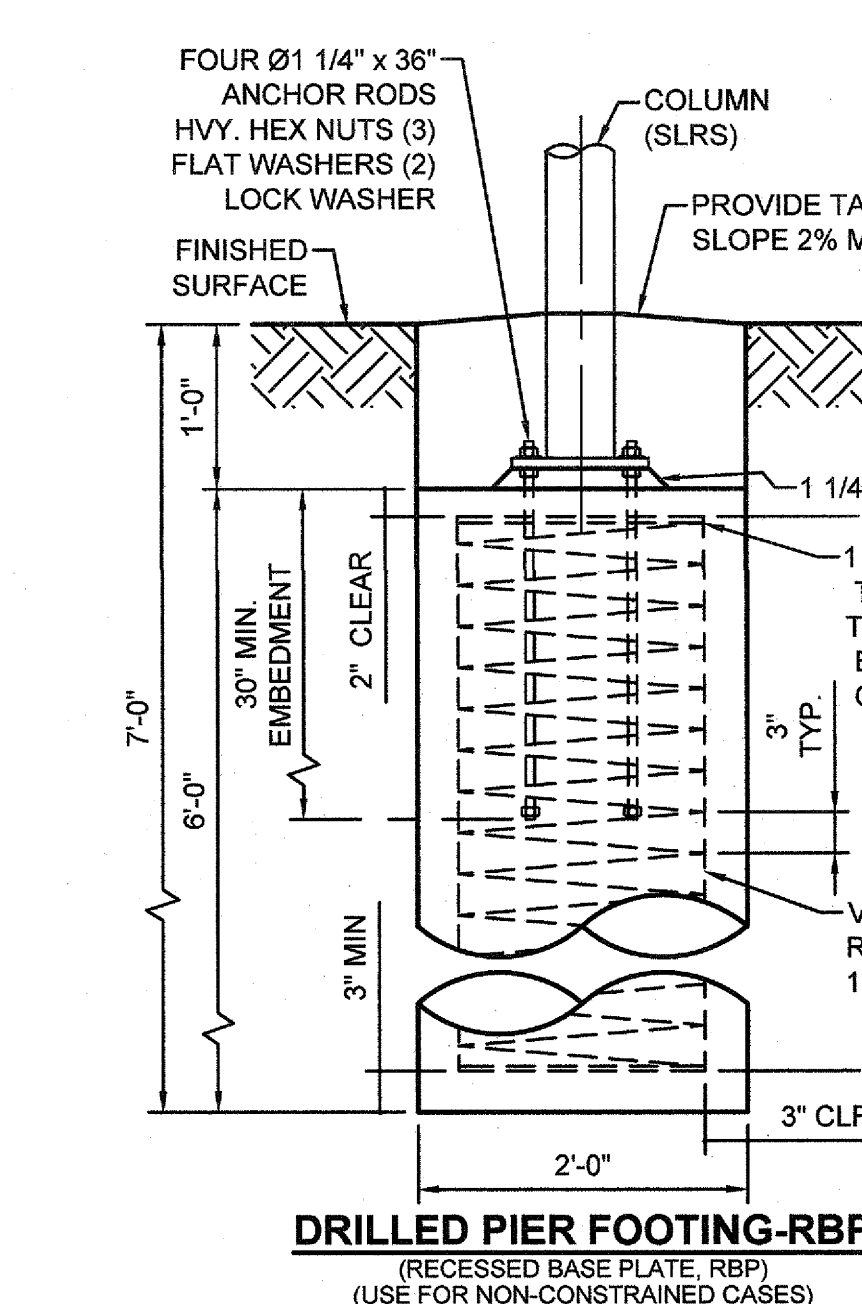
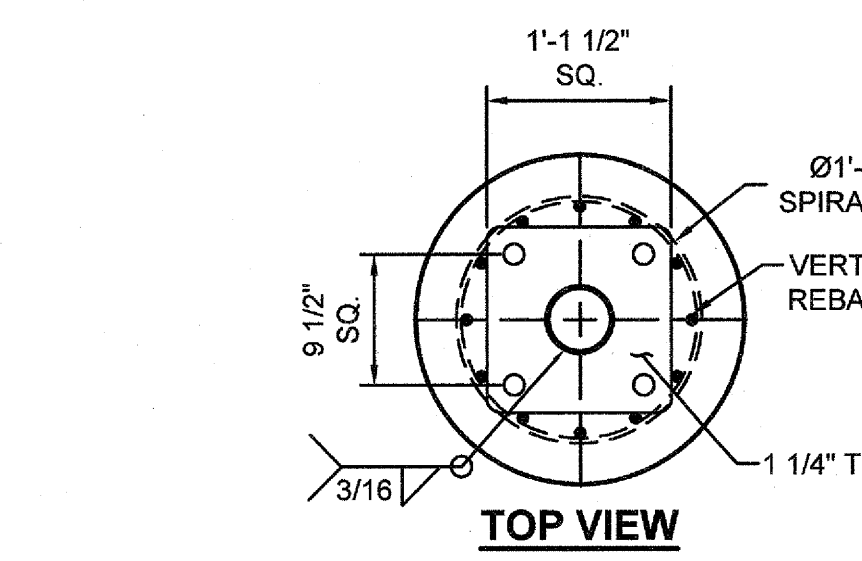
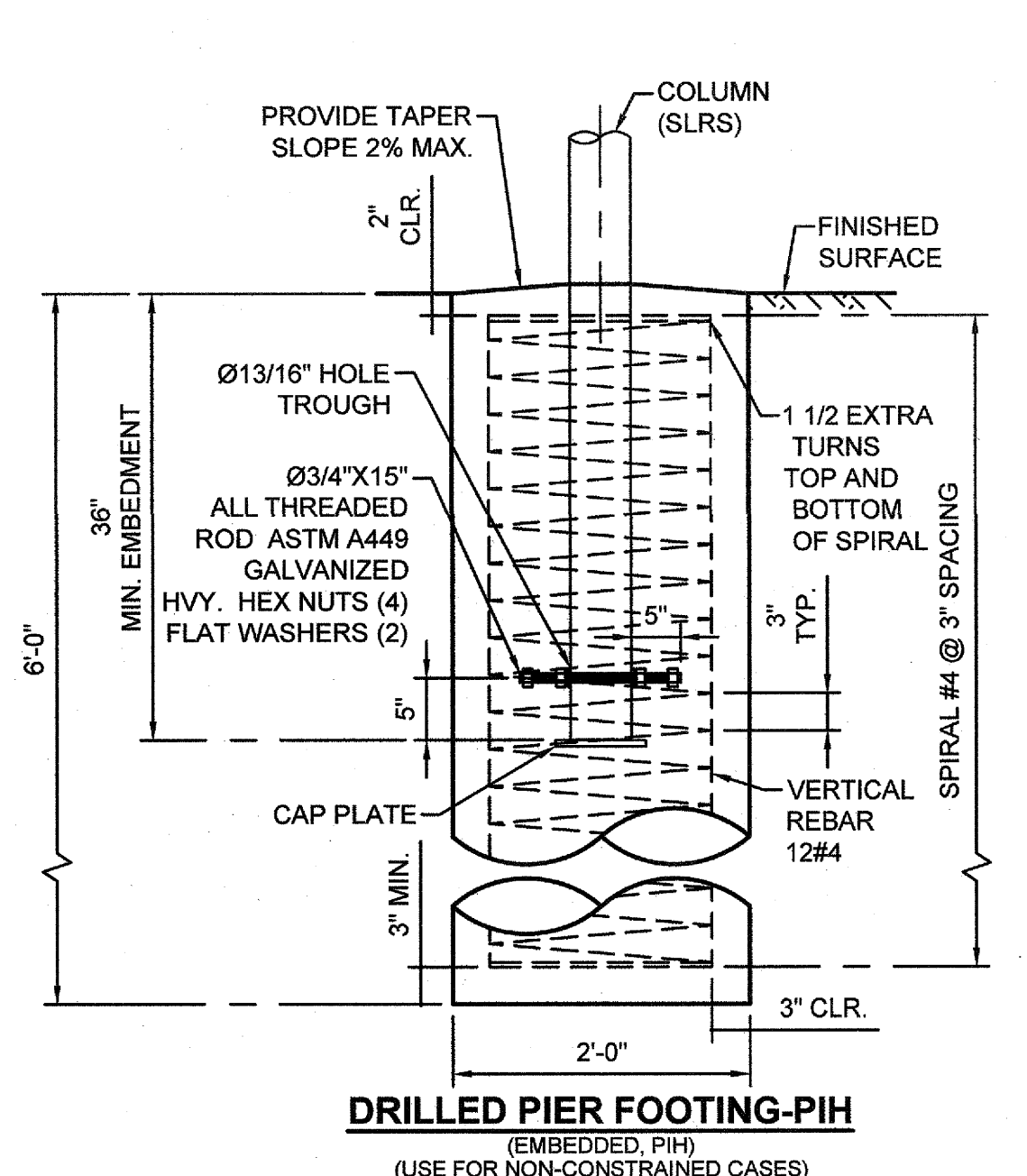
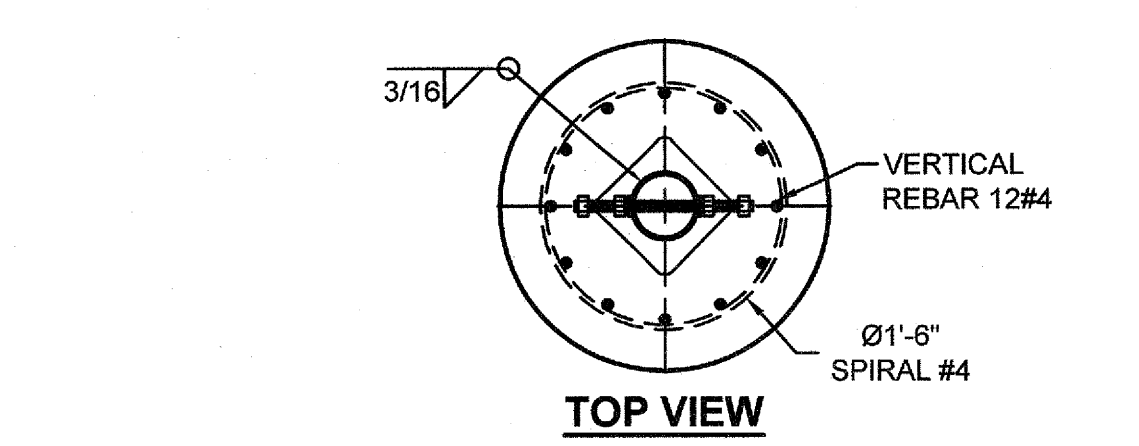
PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 14 FEET THAT INTERSECT WITH THE SLOPE (DAYLIGHTING). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIRES, A SITE-SPECIFIC SOILS REPORT IS REQUIRED.

MINIMUM CLASS 2 PROJECT INSPECTOR REQUIRED.

| CODE ANALYSIS   |           |             |                |                      |
|-----------------|-----------|-------------|----------------|----------------------|
| BUILDING        | OCCUPANCY | CONST. TYPE | AREA (SQ. FT.) | OCCUPANT LOAD FACTOR |
| SHADE STRUCTURE |           |             |                |                      |

MAXIMUM OCCUPANT LOAD (PER CBC 2016 TABLE 1604A.5)  
 -K-12: 250 PERSONS  
 -PUBLIC ASSEMBLY: 300 PERSONS  
 -EDUCATIONAL OCCUPANCIES ABOVE 12TH GRADE: 500 PERSONS



| LIST OF MATERIALS |     |                                   |                                  |
|-------------------|-----|-----------------------------------|----------------------------------|
| ITEM              | QTY | DESCRIPTION                       | MATERIAL                         |
| 1                 | 4   | COLUMN (GALVANIZED STEEL TUBE)    | 5.00 GA 7 RD. TUBE (5.0 x 0.188) |
| 2                 | 4   | CUP CONNECTOR (6" LG)             | HSS 4.0 x 0.25                   |
| 3                 | 4   | RAFTER (GALVANIZED STEEL TUBE)    | 4.50 GA 7 RD. TUBE (4.5 x 0.188) |
| 4                 | 4   | EXTENSION (GALVANIZED STEEL TUBE) | 4.50 GA 7 RD. TUBE (4.5 x 0.188) |
| 5                 | 2   | CROSSPIECE GALVANIZED STEEL TUBE) | 4.50 GA 7 RD. TUBE (4.5 x 0.188) |
| 6                 | 1   | RIDGE (GALVANIZED STEEL TUBE)     | 4.50 GA 7 RD. TUBE (4.5 x 0.188) |
| 7                 | 1   | FABRIC TOP                        | FR COLOURSHADE Z25               |
| 8                 | 1   | Ø3/8" CABLE                       | GALVANIZED STEEL                 |
| 9                 | 4   | Ø3/8" CABLE CLAMP                 | GALVANIZED STEEL                 |
| 10                | 4   | Ø5/8"-11NC x 6" HEX BOLT          | 18-8 SS                          |
| 11                | 4   | Ø5/8"-11NC HEX NUT                | 18-8 SS                          |
| 12                | 8   | Ø5/8" FLAT WASHER                 | 18-8 SS                          |
| 13                | 8   | Ø5/8" FLAT WASHER                 | DELFIN (ACETAL)                  |
| 14                | 4   | Ø5/8" SPLIT LOCK WASHER           | 18-8 SS                          |

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.



CORPORATE HEADQUARTERS  
8505-A CHANCELLOR ROW  
DALLAS, TX, 75247  
800-966-5005

CERTIFICATIONS:  
IAS CERTIFICATION No. FA-428  
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:  
Santa Monica Malibu USD

PROJECT NAME:  
Point Dume Elementary

LOCATION:  
6955 Fernhill Drive  
Malibu, CA 90265

MODEL NUMBER:  
DSA401203012-16

STRUCTURE TYPE:  
H I P  
DSA  
SIZE: MAXIMUM  
20' x 30' x 12'e MAX.  
SCALE : NONE

DRAWING SIZE: D

Professional Engineer Seal for Albert B. ... State of California, License No. 12345, Exp. 10/31/19.

Professional Engineer Seal for ... State of California, License No. 6593, Exp. 10/31/19.

FILE NUMBER: PC-SS  
IDENTIFICATION STAMP  
DIVISION OF THE STATE ARCHITECT  
APP. NO: 04 - 117219 INCR.  
AC DF FLS EA SS VJN  
DATE 09/11/2018

PRE-CHECK (PC) DOCUMENT  
Code: 2016 CBC  
A separate project application for construction is required.

Eng. By: JO 04/17/18  
Design By: MP 04/17/18  
Approved By: JO 04/17/18

DRAWING DESCRIPTION:  
PRODUCT INFORMATION

DWG. DSA401203012-16  
SHEET 20.1-1000  
REV. NC

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APPROX 11 9 2 6 6  
AC FLS SS VJN  
DATE 11 2 2018

DATE DRW CHK ENG  
DESCRIPTION  
REV



ENVELOPE JOINT REACTIONS

Shear resultant = √Px² + Py² + Pz² Moment resultant = √Mx² + My² + Mz²

Table with columns for Node, Support Forces (kip), Support Moments (kip-ft), Shear Resultant, Moment Resultant, UPLIFT, and AXIAL. Rows include ASD REACTIONS and MAXIMUM REACTIONS for nodes 84, 86, 88, 90.

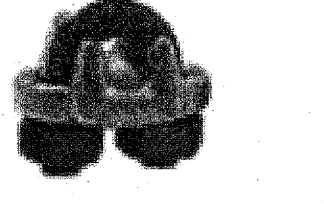
BASIC LOAD CASES

DEAD LOAD 0.0378 PSF (FABRIC)
FLOOR LIVE LOAD N/A
ROOF LIVE LOAD 5 PSF
ROOF SNOW LOAD 5 PSF
SUPERIMPOSED LOADS N/A

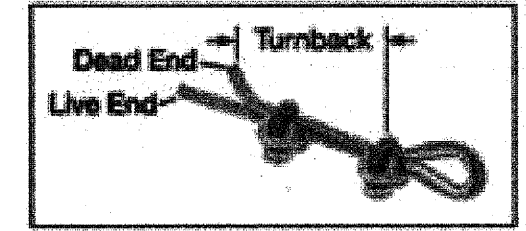
WIND LOAD
ULTIMATE DESIGN WIND SPEED (3 SEC GUST) 115 MPH
VELOCITY PRESSURE qz 24.46 PSF
COMPONENT AND CLADDING qz (CABLE AND CABLE HARDWARE ONLY) 24.46 PSF

SEISMIC LOAD
SEISMIC RESPONSE COEFFICIENTS Cs 1.8
DESIGN BASE SHEAR 1699 LB

Forged Single-Saddle Wire Rope Clamps—Not for Lifting



A forged fabrication allows these to be used in critical applications such as the down and support lines. They must be oriented with the saddle on the long (live) end and U-bolt on the short (dead) end. Also known as wire rope clips.



Aircraft Cable

Performed, made in accordance with commercial specifications military and federal specification rope available. Carbon Steel (Aircraft Cable) - Galvanized cable has the highest strength and greatest fatigue life of the materials offered.

Table for Aircraft Cable with columns for Dia. (in), Approx. Wt 1000 Ft/lbs, and Galvanized Min. Breaking Strengths (lbs).

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CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 365

CUSTOMER:
Santa Monica Malibu USD

PROJECT NAME:
Point Dume Elementary

LOCATION:
6955 Fernhill Drive
Malibu, CA 90265

MODEL NUMBER:
DSA401203012-16

STRUCTURE TYPE:
H I P
DSA
MAXIMUM
20' x 30' x 12' MAX.

SCALE: NONE
DRAWING SIZE: D



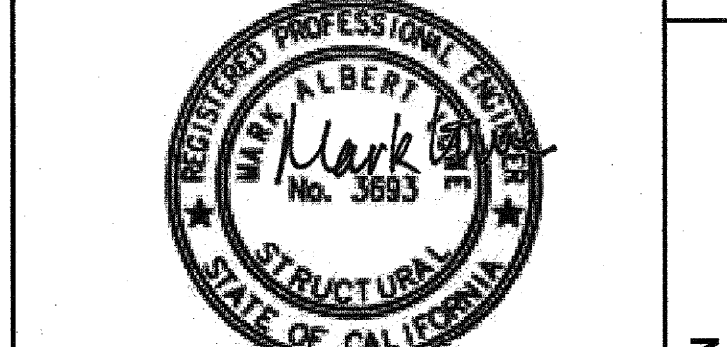
190/F5 Fire rated specifications

Standard range

Table for 190/F5 Fire rated specifications with columns for Colour, Shade %, UV Block %, Average GSM, Average Wt/break strength, Average Fibreglass %, Average Wt/break strength, Average Biogrip %, Average Bunit, Average Kpa, Average Ratio to Max ratio.

CONVERSION TO IMPERIAL UNITS:
185 GSM = .0378 psf
50 KGS = 110 lb
72 Kpa = 159 lb
156 Kpa = 3258 psf

Notes:
1. 190/F5 conforms to The California State Fire Marshal Title 19 Test for Small scale Fabrics.
2. Tests were done using a 50mm Wide strip and a cross head speed of 500mm/min.
3. No agents have been tested for water penetration or absorption in this test.



FILE NUMBER: PC-SS
IDENTIFICATION STAMP
DIVISION OF THE STATE ARCHITECT
APP. NO: 04 - 117219 INCR:
AC DF \_FLS EA \_SS \_VJN
DATE: 09/11/2016

PRE-CHECK (PC) DOCUMENT
Code: 2016 CBC
A separate project application for construction is required.

Eng. By: JO 04/17/18
Design By: MP 04/17/18
Approved By: JO 04/17/18

DRAWING DESCRIPTION:
REACTIONS
DWG. DSA401203012-16
SHEET 20.2-2000
REV. NC

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APPROX 11 9 2 6 6
AC \_FLS \_SS \_VJN
DATE: 09/21/2016