DESIGN LOADS

BUILDING CODE LIVE LOADS

5 PSF SNOW LOAD

WIND LOADS 115 MPH (3-Sec. Gust); EXPOSURE C; TOPOGRAPHIC FACTOR, Kzt = 1.0 1.- SPECIAL INSPECTION REQUIREMENTS SHALL FOLLOW THE ATTACHED SAMPLE TEST AND INSPECTION

CBC 2016 (BASED ON IBC 2015)

LIST (T & I LIST) APPROVED BY DSA. THE SHOP WELDING INSPECTION SHALL INCLUDE WELDING OF ALL STEEL MEMBERS AND IDENTIFICATION OF STEEL THROUGH MILL CERTIFICATE OR MATERIAL TESTING, UNCERTIFIED STEEL SHALL BE TESTED TO THE REQUIREMENTS OF CBC 2016 CHAPTER 17A. THE FIELD SPECIAL INSPECTION SHALL INCLUDE COMPRESSION CYLINDER TESTS FOR THE CONCRETE FOUNDATION.

2.- STRUCTURE SHALL BE IN THE LOCATION SHOWN ON THE SITE SPECIFIC DSA APPLICATION DRAWING.

3.- FOUNDATION DESIGN BASED ON CBC 2016, TABLE 1806A.2, SOIL CLASS 5 (ALLOWABLE FOUNDATION PRESSURE 1500 PSF)

4.- DESIGN PER FOLLOWING CODES: CBC 2016, ASCE 7-10, AISC 360-10, AISC 341-10, ACI 318-14, ASCE 55-10 & ASCE 19-10

STRUCTURAL STEEL

1.- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL BE CONDUCTED PER CBC 2016 SECTIONS 1704A, 1705A, 1705A.2, AND TABLE 1705A.2.1.

2.- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE SHADE STRUCTURES.

4.- ALL GALVANIZED STEEL TUBE PRODUCTS MANUFACTURED BY ALLIED TUBE & CONDUIT FOR THIS STRUCTURE SHALL BE, AND CONFORM TO ASTM A500-10: GRADE "B", IN ITS' ENTIRETY.

TYPICAL MECHANICAL PROPERTIES ARE: **ROUND TUBE** 42,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS

5.- ALL STRUCTURAL SHAPES SHALL BE COLD FORMED HSS ASTM A500 GRADE B, UNLESS OTHERWISE NOTED. TYPICAL MECHANICAL PROPERTIES ACHIEVED FOR HSS PRODUCTS: SQUARE AND RECTANGULAR 46,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS ROUND PIPE 42,000 PSI YIELD STRESS / 58,000 PSI TENSILE STRESS

6.- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.

7.- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.

8.- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/CWI INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COLD FORMED. AWS D1.8 SEISMIC SUPPLEMENT.

9.- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.

10.- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. FIELD CONNECTIONS SHALL BE AS INDICATED ON THE DRAWINGS (IF REQUIRED). ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" ER70SX ELECTRODES UNLESS OTHERWISE NOTED. EITHER SMAW OR GMAW IS ACCEPTABLE.

1 OR 2 ALL NUTS SHALL COMPLY WITH ASTM F-594 ALLOY GROUP 1 OR 2. REFERRING TO RCSC, ASTM F-593 IS NOT CONSIDERED AS HIGH STRENGTH BOLTS. 12.- ALL STRUCTURAL STEEL (ITEMS FROM NOTE 5) SHALL BE PAINTED WITH ONE SHOP COAT (2.5 TO 3.5

MILS THICK MIN) OF ZINC-RICH PRIMER. UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER TGIC (MANUFACTURED BY SHERWIN WILLIAMS OR TIGER DRYLAC). TO ACHIEVE OPTIMUM ADHESION, IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TGIC) SPECIFICATIONS SHALL BE AS FOLLOWS: - PENCIL HARDNESS (ASTM D-3363).

- HUMIDITY (ASTM D-2247). - SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL. SOFTNESS.

13.- ALL STEEL ROUND TUBING (ITEMS FROM NOTE 4) SHALL BE TRIPLE COATED FOR RUST PROTECTION USING THE IN-LINE ELECTROPLATING COAT PROCESS. TUBING SHALL BE INTERNALLY COATED WITH ZINC AND ORGANIC COATINGS TO PREVENT CORROSION AS MANUFACTURED BY ALLIED TUBE & CONDUIT.

14.- COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION. ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM) HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329), OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

CONCRETE SPECIFICATION

1.- CONCRETE SHALL BE TESTED PER CBC 2016 SECTION 1903A & SHALL BE INSPECTED PER SECTION

2.- CONCRETE TO BE F'c= 4500 PSI, TYPE V CEMENT, WATER/CEMENT RATIO OF 0.45, PER ACI 318-14 CHAPTER 5. REINFORCING STEEL TO BE Fy= 60000 PSI, MIN. GR. 60

3.- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 55 (GALVANIZED). ANCHOR BOLT'S EMBEDMENT NEEDS TO BE AS FOLLOW: A) ANCHOR BOLT Ø1 1/4" 30 IN (MINIMUM EMBEDMENT)

4.- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.

5.- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.

FABRIC SPECIFICATION

1.- FABRIC SHALL BE MANUFACTURED BY MULTIKNIT LTD. OR OTHER COMPANY WHO CAN MANUFACTURE FABRIC, WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS.

2.- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR

3.- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHALL TO DSA AT SITE SPECIFIC INSTALLATION.

4.- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT. FABRICS SAMPLES OF THE SAME MATERIAL WHICH ARE MAINTAINED AT THE PROJECTS SITE SHALL BE TESTED TO BE IN COMPLIANCE WITH ASTM D5034 AND D2261. THE ANNUAL TESTING ON THE APPROVED PLANS SHALL BE COMPARED TO THE FABRIC SPECIFICATIONS INDICATED IN NOTE 1 OF "FABRIC SPECIFICATION" ON THE APPROVED PLANS. THE FABRIC SHALL BE REPLACED WHEN THE TEST RESULTS RETURN LESS THAN 50% OF THE ULTIMATE VALUES IN NOTE 1 OF "FABRIC SPECIFICATION".

5.- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED, FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.

6.- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. SHADE STRUCTURE SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

AIRCRAFT CABLE

1.- FOR FABRIC ATTACHMENT USE 3/8" 7x19 GALV. CABLE PER ASTM A1023A, ASTM 1023M-02, WITH A BREAKING STRENGTH VALUE OF 14,400 LBS. CABLE SHALL BE TENSIONED TO 250 LBS MINIMUM. THE MAXIMUM CALCULATED CABLE TENSION IS 2189 LB.

2.- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTING VISITS AS REQUIRED.

2016 CBC PC DESIGN NOTES

SYSTEM.

FLOOR LIVE LOAD RLL 5 PSF ROOF LIVE LOAD **ALLOWABLE SOIL PRESSURE:** 1500 PSF DL + LL (CONC FTG)

DL + LL + SEISMIC (CONC FTG) 1500 PSF 100 PSF/FT BELOW NATURAL LATERAL BEARING DESIGN VALUE GRADE, PER TABLE 1806A.2 TWO TIMES THE TABULAR VALUE IS USED (200 PSF/FT)

PER CBC SECTION 1806A.3.4. ALLOWABLE PIER FRICTIONAL RESISTANCE 250 PSF MAXIMUM BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE). UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.

ROOF SNOW LOAD

FLOOD HAZARD AREA WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X. A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.

WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-10, SECTION 27.4.3 -ULTIMATE DESIGN WIND SPEED (3 SEC GUST) 115 MPH -WIND EXPOSURE FACTOR -TOPOGRAPHIC FACTOR -RISK CATEGORY -VELOCITY PRESSURE EXPOSURE COEFFICIENT 0.85 24.46 PSF -VELOCITY PRESSURE 3.- ALL WORK SHALL CONFORM TO CBC 2016 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR) SEISMIC DESIGN:

-SITE CLASS 3.00g 1.389g 2.00 SDS -SPECTRAL RESPONSE COEFFICIENTS -LATERAL FORCE RESISTING SYSTEM G.2 ORDINARY CANTILEVERED COLUMN

-SEISMIC IMPORTANCE FACTOR 1.0 -DESIGN BASE SHEAR 1699 LB -SEISMIC RESPONSE COEFFICIENTS Cs 1.6 1.25 -RESPONSE MODIFICATION FACTOR **EQUIVALENT LATERAL FORCE** -ANALYSIS PROCEDURE -RISK CATEGORY -SEISMIC DESIGN CATEGORY -SITE COEFFICIENT CATEGORY

GEOHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQF OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3.1.1 DO NOT REQUIRE A GEOHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

Fν

11.- ALL STAINLESS STEEL BOLTS SHALL COMPLY WITH ASTM F-593, FYS = 60 KSI, FS = 95KSI ,ALLOY GROUP ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

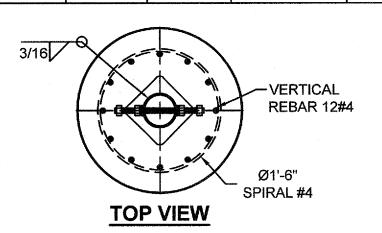
> PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE

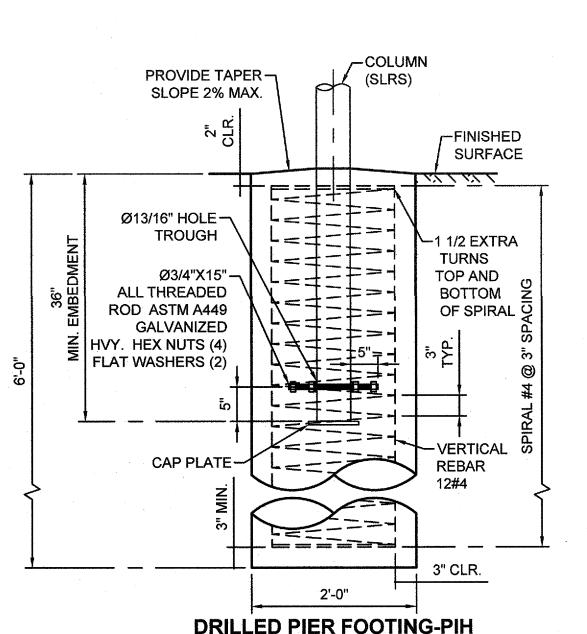
MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 14 FEET THAT INTERSECT WITH THE SLOPE (DAYLIGHTING). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIRES, A SITE-SPECIFIC SOILS REPORT IS REQUIRED.

UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

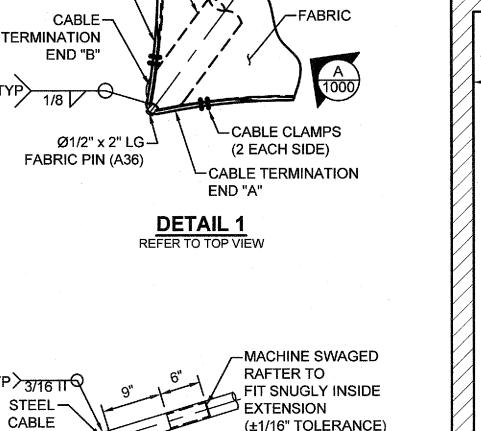
MINIMUM CLASS 2 PROJECT INSPECTOR REQUIRED.

CODE ANALYSIS						MAXIMUM OCCUPANT LOAD (PER CBC 2016 TK-12: 250 PERSO		
BUILDING	OCCUPANCY	CONST.	AREA (SQ. FT.)	OCCUPANT LOAD FACTOR	OCCUPANT LOAD	-PUBLIC ASSEMBLY: -EDUCATIONAL OCCUPANCIES	300 PERSON	
SHADE STRUCTURE						ABOVE 12TH GRADE:	500 PERSON	
							1'-1 1/2" SQ.	





(USE FOR NON-CONSTRAINED CASES)



-3/8" THK STIFFENER

-CUP CONNECTOR

TO CUP

TO COLUMN

Ŏ_{1/4} ✓ TYP CAP PL

O_{3/16} / TYP CAP PL

VIEW-A

REFER TO DETAIL-

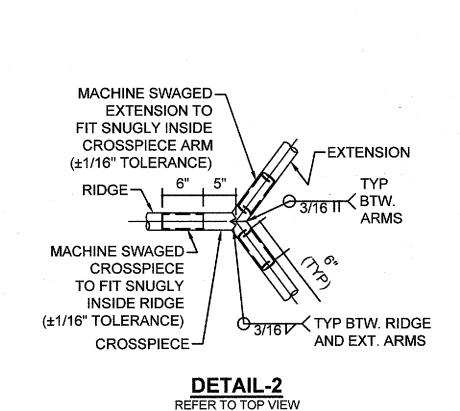
STEEL CABLE-

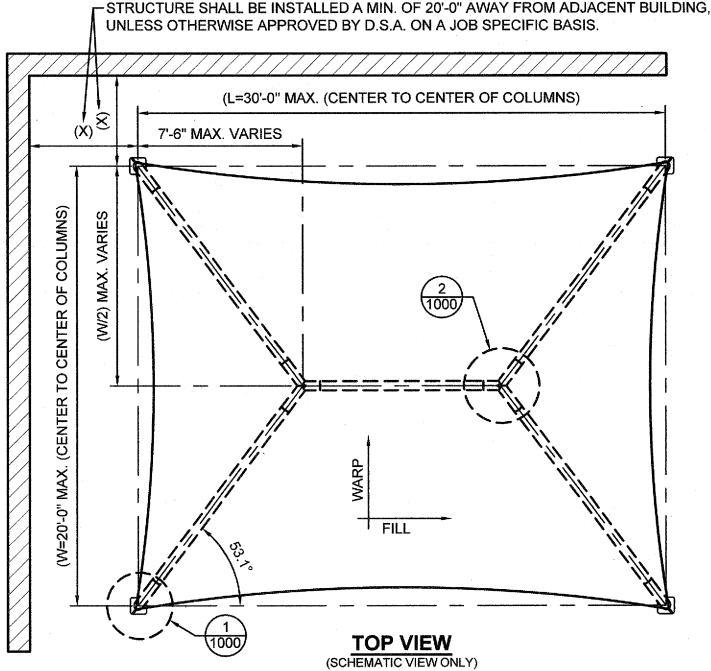
RAFTER

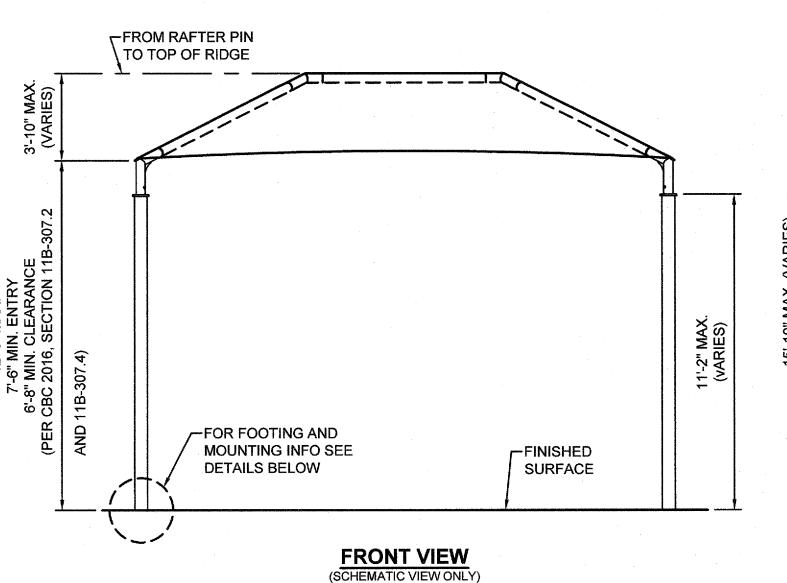
10,11,12,13,14

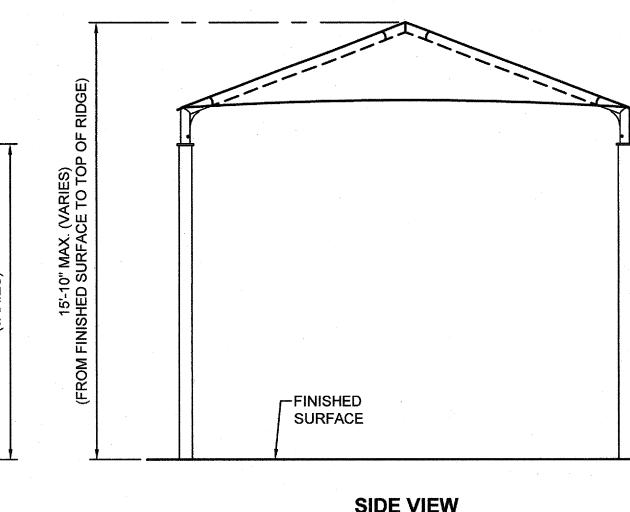
3/4" THK PL-

(Ø11/16" HOLE)









LIST OF MATERIALS

MATERIAL

5.00 GA 7 RD. TUBE (5.0 x 0.188)

HSS 4.0 x 0.25

FR COLOURSHADE Z25

GALVANIZED STEEL

GALVANIZED STEEL

18-8 SS

18-8 SS

18-8 SS

DELRIN (ACETAL)

18-8 SS

DESCRIPTION

COLUMN (GALVANIZED STEEL TUBE)

CUP CONNECTOR (6" LG)

RAFTER (GALVANIZED STEEL TUBE)

EXTENSION (GALVANIZED STEEL TUBE)

CROSSPIECE GALVANIZED STEEL TUBE)

RIDGE (GALVANIZED STEEL TUBE)

FABRIC TOP

Ø3/8" CABLE

Ø3/8" CABLE CLAMP

Ø5/8"-11NC x 6" HEX BOLT

Ø5/8"-11NC HEX NUT

Ø5/8" FLAT WASHER

Ø5/8" FLAT WASHER

Ø5/8" SPLIT LOCK WASHER

ITEM QTY

9 4

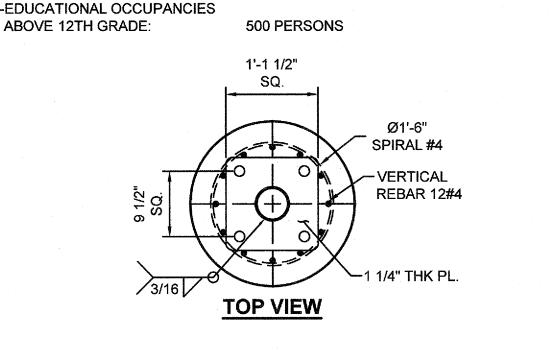
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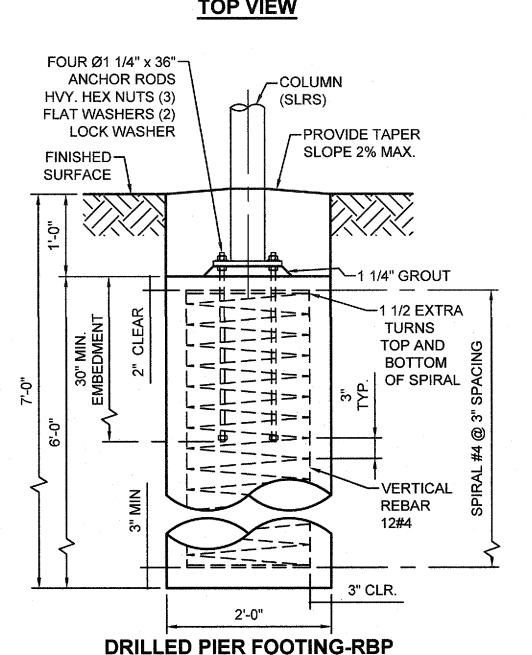
14

4

8

SIDE VIEW

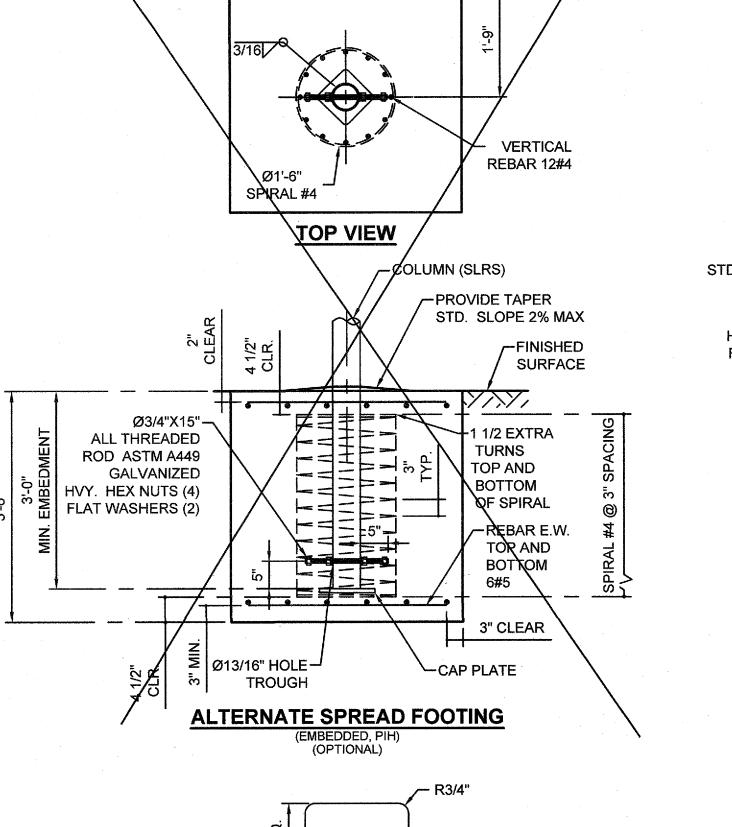




RECESSED BASE PLATE, RBF

(USE FOR NON-CONSTRAINED CASES)

(OPTIONAL)

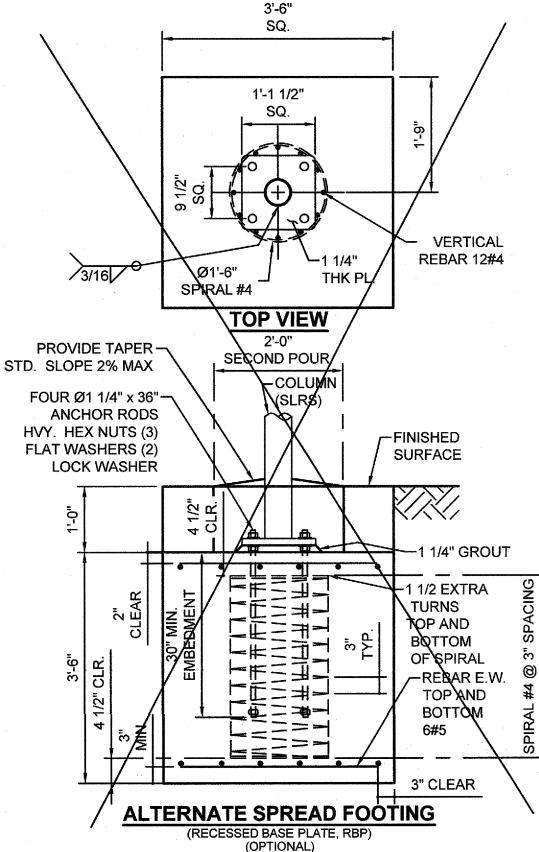


CAP PLATE

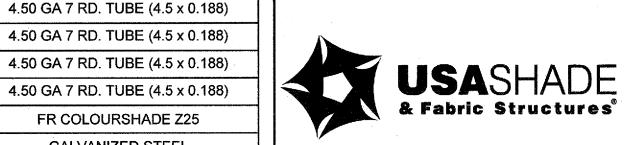
(TYP. FOR ALL COLUMNS)

(TOP OF RBP COLUMNS)

(TOP & BOT. OF PIH COLUMNS)



IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP03 1 1 9 2 6 6 AC___FLS_7C_SS__FT DATE___UEU__2_1_2018 THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.



CORPORATE HEADQUARTERS 8505-A CHANCELLOR ROW DALLAS, TX, 75247 800-966-5005

CERTIFICATIONS: IAS CERTIFICATION No: FA-428 CLARK COUNTY MANUFACTURER

CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER: Santa Monica Malibu USD

PROJECT NAME: |Point Dume Elementary

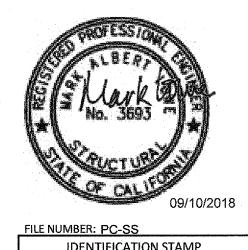
LOCATION: 6955 Fernhill Drive Malibu, CA 90265 MODEL NUMBER:

DSA401203012-16 STRUCTURE TYPE:

DSA **MAXIMUM** 20' x 30' x 12'e MAX.

SCALE: NONE

DRAWING SIZE:



IDENTIFICATION STAMP **DIVISION OF THE STATE ARCHITECT** APP. NO: 04 - 117219 INCR: AC DF FLS EA SS VJN DATE 09/11/2018

PRE-CHECK (PC) Code: 2016 CBC A separate project application for construction is required.

Eng. By: 04/17/18 Design By: 04/17/18 Approved By: JO DRAWING DESCRIPTION:

PRODUCT INFORMATION

DSA401203012-16

20.1-1000

NC

ENVELOPE JOINT REACTIONS

Shear resultant = $\sqrt{Px^2 + Py^2 + Pz^2}$

 $Moment resultant = \sqrt{Mx^2 + My^2 + Mz^2}$

			The second secon				ASD REACTION	DNS				
Hode	L Company		Support Porces (kg	A SECTION OF THE SECTION OF THE SEC	Support No	muota (kijaht)			Support Forces (kip)	Support Morners (kipft)	Support Forces (kip)	Support Forces (M)
No.		4,	1	P _r	H.	I A,	W _E		SHEAF RESULTANT	MORMENT RESULTANT	URLET	ZAMA
						L BAAY	I IMUM REAC	TIANE	0.775	6.238	0.585	-1.380
					and the material and the second secon				0.773	V-4-10		-1:300
Hoses,			Support Poster (6)		5.0	ne a la fericament i i i [] :	ofij					
No.		P _c	\$	P _i	W,	W _t	My					
84	Мак	0.720	0.547	0.584	0.842	5.631	0.395				and a second and a	
	Min	-0.258	-0.208	-1.338	-4.534 -0.963	-2.324 5.631	-0.926 -0.926	CO 5	0.720	5.713	0.584	
	Max P _a Min P _a	0.720 -0.258	-0.008 0.547	0.584 -0.935	4,534	-2.324	0.315	6032	0.605	5.095		-0.935
	Max P.	-0.258	0.547	-0.935	-4.534	-2.324	0.335	CO 32	0.605	5.095		1.934
	Min P,	0.623	-0.208	0.301	0.842	4.416	-0.438	CO 4	0.657	4.496	0.301	
	Max P ₂	0.720	-0.008	0.584	-0.963	5.631	-0.926	CO 5	0.720	5.713	0.584	
	Man P ₂	-0.229	0.485	-1.335	-3,2,14	41.776	0,136	CO 11	0.536	4.207		-1.338
= 1	Max M _x	0.623	-0,208	0.301	0.842	4.416	-0.438	604	0.657	4.496	0.301	
	Min M _a	-0.258	0.547	4.935	4.33	-2,324	0.335	CO 33	0.605	5.095		-0.935
	Max M _y	0.720	-0.008	0.584	-0.963	5.631	-0.926	EQ 5	0.720	5.713	0.584	
	Min M _y	-0.258	0.547	-0.935	4.534	-2.324	0.385	CO 32	0.605	5.095		-11.935
	Max M ₁	-0.258	0.547	-0.935	4534	-2.324	0.326	CO 32	0.605	5.095		-0.935
	Min M ₂	0.720	-0.008	0.584	4.363	5.631	-0.926	605	0.720	5.713	0.584	
286	Max	0.623 -0.029	0,631 -0,131	0.570 -1.380	0.944	5.714	1.499 -0.240					Paratura por la seria de sala un esta desconada de equanda de esta de esta de esta de esta de esta de esta de e
	Min Mar P _e	0.623	0.508	0.301	-0.842	4.417	0.438	CO 4	0.657	4,497	0.301	
	Win P.	-0.029	-0.125	-0.797	0.867	0,099	0.228	60 30	0.128	0.873		-0.797
	Max P.	0.450	0.631	0.570	4.227	2.550	-0.037	60.84	0.775	4.937	0.570	
	Min P _x	-0.001	-0.131	-0.931	0.944	0.352	0.291	CO 1	0.131	1.007		40,931
	Man F ₂	0.450	0,631	0.570	-4(227	2.530	-0.037	CO 34	0.775	4.937	0.570	
	Min P ₃	0.365	0,140	-1.340	-1,084	3.903	1.227	60 11	0.391	4,051		-1,300
and the second	Mas M _s	-0.001	-0,131	-UPBI	0.944	0.352	0.291	CO 1	0.131	1,307		-0.931
	Min M _s	0.450	0.631	0,570	4.27	2,550	-0.087	CO 34	0.775	4.937	0.570	
	Max M _t	0.599	0.352	-0.969	-2.503	5,714	1.499	CO 3	0.695	6.234		-0.969
	Min M _i	0.000	0.000	0.000	0,000	0.000	0.000		0.000	0.000		
	Max M ₂	0.599	0.352	40.969	-2,503	5,714	1,499	CO 3	0,695	6.238		-0,969
	Min M _g	0.282	0.058	-0.839	-0.586	2.562	-0.280	6031	0.288	2.628		-0.839
93	Max Win	0.329 -0.599	-0.162	0.569 -1.380	1.307	2.744 -5.712	0.116 -1.500					
	Max P _e	0.329	-0.033	-0.428	0.300	2,744	-0.301	CO 16	0.331	2.760		-0.428
2.020	Min P _e	-0.599	0.353	-0.469	-2.505	-5.712	-1.500	CO 3	0.685	6,237	organistica processor processor supplementations in construction of excessor and administration of excessor and excessor a	-0.969
	Max P _e	-0.450	0.630	0.560	-4.221	-2.547	0.086	CD 34	0.774	4.530	0.569	
	Min P _v	0.135	-0.162	-1.103	1.307	0.910	-0.530	CO 10	0.211	1.593		*1.103
	Max P _z	-0.450	0.630	8.562	-4.221	-2.507	0.086	60.24	0.774	4.930	0.569	
	Man P _a	-0.364	0,141	-14:30	-1.086	-3.501	-1337	CO 11	0.590	4,049		-1.380
	Max M _s	0,135	-0.162	-1.103	1.307	0.910	-0.530	CO 10	0.211	1.593		-1.103
	Min M _s	-0.450	0,530	0.569	4.221	-2,547	0.086	CO 34	0.774	4.530	0.589	
	Max M ₁	Q.329	-0.033	-0.428	0.300	2.744	-0.20L	CO 16	0.331	2,760		-0.428
	Min M _r	-0.599	0.353	-0.969	-2.505	-5.712	-1.500	003	0.695	6.237	A. A. W. W.	-0.969
	Max M ₂	-0.259	0.354	0.465 -0.969	-2.174 -2.505	-1.438 -5.712	0.116 -1.500	CO 4	0.439 0.695	2.607 6.237	0.465	-0.969
90	Min M ₂ Max	-0,599 0,329	0.353	0.585	2.173	2.744	0.928		V.V.852			70,503
74	Min	-0.721	-0.354	-1.337	-4.534	-5.630	-0.394					
	Max P _i	0.379	0.033	-0.428	-0.300	2,744	0.201	CO 16	0.331	2,760		-0.428
	Min P _E	-3.721	-0.00%	0,585	-0.984	-5.630	0.928	co s	0.721	5.712	0.585	
	Max P,	0.258	0.547	-0.934	-4534	2.325	-0.394	CO 32	0.605	5.035		-0.0120
	Män P _e	-0.259	-0.358	0,465	2.173	-1,437	-0.116	CO 4	0.439	2.665	0.465	
	Max P _i	-1744	-0.008	0,585	-0.964	-5,630	0.928	CD 5	0.721	5,712	0.585	
	Min P _t	0.229	0,485	-1.337	-3,813	1,777	-0.135	60 11	0.536	4.207		-1.337
	Max M _s	-0.259	-0,354	0.465	2.173	-1.437	-0.116	004	0.439	2.605	0.465	
	Min M _a	0.258	0.547	-0.934	454	2,325	-0.394	[032]	0.605	5.095		-0,934
	Max M _Y	0.329	0.033	-0.428	-0.300	2,744	0.201	CO 16	0.331	2.760		-0,428
	Min M _y	-0,721	-0.008	0.585	-0.964	-5.630	0.928	COS	0.721	5.712	0.585	

BASIC LOAD CASES

9

DEAD LOAD 0.0378 PSF (FABRIC) FLOOR LIVE LOAD

ROOF LIVE LOAD 5 PSF ROOF SNOW LOAD 5 PSF SUPERIMPOSED LOADS

ULTIMATE DESIGN WIND SPEED (3 SEC GUST) 115 MPH VELOCITY PRESSURE qz COMPONENT AND CLADDING qz

(CABLE AND CABLE HARDWARE ONLY) 24.46 PSF SEISMIC LOAD

1.6 1699 LB SEISMIC RESPONSE COEFFICIENTS Cs DESIGN BASE SHEAR

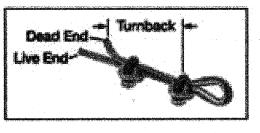
Forged Single-Saddle Wire Rope Clamps—Not for Lifting



A forged fabrication allows these to be used in critical applications such as the downs and support lines. They must be oriented with the saddle on the long (live) and and U-bolt on the short (dead) and. Also known as wire rope clips Galvanized steel clamps have a thick coating for corresion resistance.

316 stainless steel clamps are the most conosion resistant fittings we offer. They provide excellent resistance to sait water and

Warning: Test all assemblies for required strength before use. Do not use with coaled rope unless the coaling is removed.



					-Сили-			
For Ruge Dia	No. of Clamps Required	Rugas Turntas k	Required Torque, 11-lbs.		Wd.		Capacity	Specifications Well
Salvantzed	Steel	ann de la descripción de la Marie de La descripción de la Marie de la descripción de la Marie de la descripción	MATERIAL PROPERTY AND	action of the second section of the second second	Motorcoad Metal Annaeroph of Charles International	MAN BOOTET THE SCORE SERVICE S	TERCHINATION BERNETTECO TREAD TREADER OF THE SECOND TREATMENT AND THE S	
LE"		3 197	4.5	1 1/10	1	12/19	80% of the Rope's Capacity	Septiment:
376	2	334	7.5	1 1/2"	1210	1 P4	80% of the Rope's Capacity	Tablemaik I
1/4	2	4 24"	15	1 3/4"	1 7716	1 174	80% of the Rope's Capacity	Fed Spec FF-C-450
576"	2	514	30	2 1/8"	1 11/16	1 5/16	80% of the Rope's Capacity	FEG Spec FF-C-450
102 m	4	B 12°	45	27/16	2	1 ters	80% of the Rope's Cauachy	Fec. Spec. FF-C-450
7/10	2	1.		3 tine	2510	1 (6)16	即落 Millie Rope's Capacity	FBIL Space FF-1-451
13"	3	11 12"	65	3 une	2010	1 12 10"	80% of the Rope's Capacity	Fed Spec FF-C-450
grie"	3	12	95	350	210	21/10	80% of the Rope's Capacity	Fed. Spec. FF-C-450
46	3		95	355	210	2 1/16"	80% of the Rope's Capacity	Fed Stee FF-C-450
14	4	14	130	4 2/15	2 798	2 1/4"	80% of the Rope's Capacity	Fed Spac FF-C-450
T/M"	4	19*	325	4 2/4	1111	2 10	80% of the Rope's Capacity	Fed Scent FF-C-450
in and the second	5	35	25	55%	317	2 11/18	90% of the Roge's Capacity	FIND STORE FF-C-450
1.6	6	34*	225	5 12/18"	350	2 (3/65"	90% of the Rope's Capacity	Fed Stet FF-C-450
14	7	44*	380	6 5 8	4 2/10	3 210	90% of the Rope's Capacity	Fed Spec FF-0-450
116"		44*	360	6 24"	4 114	3 200	90% of the Rope's Capacity	Fed Spec FF-C-450
1 +2"		547	300	7 Tite	4 14	3 7/16	190% of the Roce's Capacity	Fed. Spec. FF-C-450

Aircraft Cable

Preformed, made in accordance with commercial specifications military and federal specification rope available.

Carbon Steel (Aircraft Cable) - Galvanized cable has the highest strength and greatest fatigue life of the materials offered. It has good to fair corrosion resistance in rural to industrial atmosphere environments. This material is most widely used for small diameter cables. Tin over galvanized cable offers greater corrosion resist-ance and reduced friction over pulleys.

	71	Gairanized Min.		
	Dia. (In)	Approx. Wi 1000 Ft/lbs	Breaking Strengths (ib:	
	8/82	17.	1,000	
	1/8	20.	2,000	
7 x 19	5/32	45.	2,800	
	3/16	65.	4,200	
	7/32	86.	5,600	
	1/4	110.	7,000	
	9/32	139.	8.000	
	5/16	173	9,800	

190/F5 Fire rated specifications Standard range CONVERSION TO May to IMPERIAL UNITS: Shade % UV Block % 185 GSM = .0378 psf 50 KGS = 110 Lb 0,94 72 KGS = 159 Lb 150 0.64 156 Kpa = 3258 psf 156 .0.84 Terracella 110 LB 159 LB 190/FS conforms to The California State Fine Marshal Title 19 Text for Small scale Fabrics That tests are done using a Stanm Wide strip and across read speed of 500mm/mms The properties that are considered, and the properties of months from the properties of the constitution o to be a resolution of the lateral properties of the United States of the United States of the Control of the Co Company the region best engaged ble at label to any way absoluted a training and a training at a region of the stay built at the

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.



CORPORATE HEADQUARTERS

8505-A CHANCELLOR ROW DALLAS, TX, 75247 800-966-5005

CERTIFICATIONS:

IAS CERTIFICATION No: FA-428 CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:

Santa Monica Malibu USD

PROJECT NAME:

Point Dume Elementary

LOCATION:

6955 Fernhill Drive Malibu, CA 90265

MODEL NUMBER: DSA401203012-16

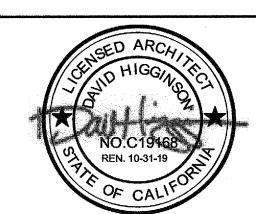
STRUCTURE TYPE:

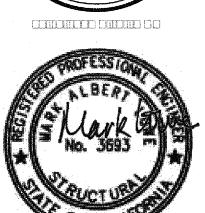
HIP DSA

MAXIMUM 20' x 30' x 12'e MAX.

SCALE: NONE

DRAWING SIZE:





FILE NUMBER: PC-SS IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITECT APP. NO: 04 - 117219 INCR:

AC DF FLS EA SS VJN DATE 09/11/2018

PRE-CHECK (PC) DOCUMENT Code: 2016 CBC A separate project application for construction is required.

Eng. By :	JO	04/17/18
Design By :	MP	04/17/18
Approved By :	JO	04/17/18
DRAWING DESC	CRIPTION:	

REACTIONS

DSA401203012-16

20.2-2000

NC

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT